

DATE: April 14, 1994

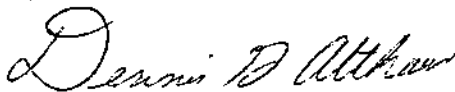
TO: Richard J. Pauser
Construction and Materials Supervisor
Transportation District 6

FROM: Bruce J. Pfister, P.E.
Chief Geotechnical Engineer

SUBJECT: Site Investigation Report
Project I.D. 8651-00-00
STH 128 Over South Branch of Tiffany Creek
Spring Valley to Glenwood City
Structure B-55-158
St. Croix County

We are attaching copies of a Site Investigation Report for the project noted above.

by:



Dennis G. Althaus

JBP:DGA:m00174
Attachments

cc: District 6 (orig. +3)
C.O. Bridge (2)
C.O. Files
C.O. Design
J.E. Haverberg
Geotechnical File

**SITE INVESTIGATION REPORT
PROJECT I.D. 8651-00-00
STH 128 OVER SOUTH BRANCH OF TIFFANY CREEK
SPRING VALLEY TO GLENWOOD CITY
STRUCTURE B-55-128
ST. CROIX COUNTY**

1. GENERAL

Two borings were made for a proposed single span structure to replace the present single steel girder structure. The proposed structure will carry STH 128 over the south branch of Tiffany Creek at station 349+89±. The proposed structure is offset 10 feet to the south of the present structure. The structure is located 1.1 miles north of the STH 128 and CTH "DD" intersection. There is some cracking at the abutment walls but overall the existing structure is in good condition. The bituminous pavement shows some cracking but is in generally good condition. The 5 foot approach fills look to be in good condition. There is some erosion on the creek banks. Rolling hills have farm fields and woods for ground cover. Tiffany Creek is 4 to 10 feet wide and 4 feet deep at the structure. No rock outcrops, boulders or marsh areas were noted. The surface soil is a sandy loam.

2. SUBSURFACE CONDITION

Two borings were made conforming to AASHTO Method T-206, Standard Penetration Test, to estimate relative density, fix presumptive bearing capacity, investigate soil properties to select suitable pile types with their support values, make a cursory review of alternate foundation possibilities, and recover samples for soil textural identification and classification. Soil textures noted on the drilling logs are drillers field identification with a later check at the Central Geotechnical Section Office.

Boring 1 was taken at station 350+08 26 feet left of the existing centerline. Boring 2 was logged as the following; elevation 1014.3 to 974 firm gray/brown sand with a trace of gravel and silt, 974 to 965 dense to very dense brown sand with some gravel and a trace of silt, 965 to 954 very dense greenish shale (rock).

The water elevation at the time the borings were made was 1006.2.

Rock elevation was 964± for boring No. 2.

3. BEARING CAPACITY

A cursory review indicates that the soils down to a practical footing depth are inadequate for bridge support on footings.

4. PILES

The following chart can be used to set pile lengths.

Structure Unit		End Bearing psf	
Boring No.	Skin Friction (psf)	(SF=2)	
<u>Elevation</u>	<u>(SF=2)</u>	<u>(Displ.)</u>	<u>(H-Piles)</u>
South Abutment			
Boring 2			
1014 to 1006	50	--	--
1006 to 974	300	--	--
974 to 965	600	79,000	47,000
965 to 954	Rock	Refusal	
North Abutment			
Boring 1			
1006 to 994	50	--	--
994 to 972	300	44,000	26,000
972 to 960	950	240,000	147,000

H-piles or oil field pipe piles however could be driven to 9000 psi load in the steel section if driven to rock/weathered rock. The estimated pile tip elevations would be 965 ± for both abutments.

5. ALTERNATE FOUNDATION TYPE

Drilled caissons could be used here but the cost would be more. Dynamic and vibratory methods could not be used effectively here.

6. LATERAL EARTH PRESSURE

The active lateral earth pressure can be held to a minimum of 30 to 35 psf if the fill material behind the abutment or other earth retaining structures is a good sand material and adequate compaction and thorough drainage is maintained. If a silty sand material is used increase the pressures to 45 psf or more.

7. CONSTRUCTION PROBLEMS

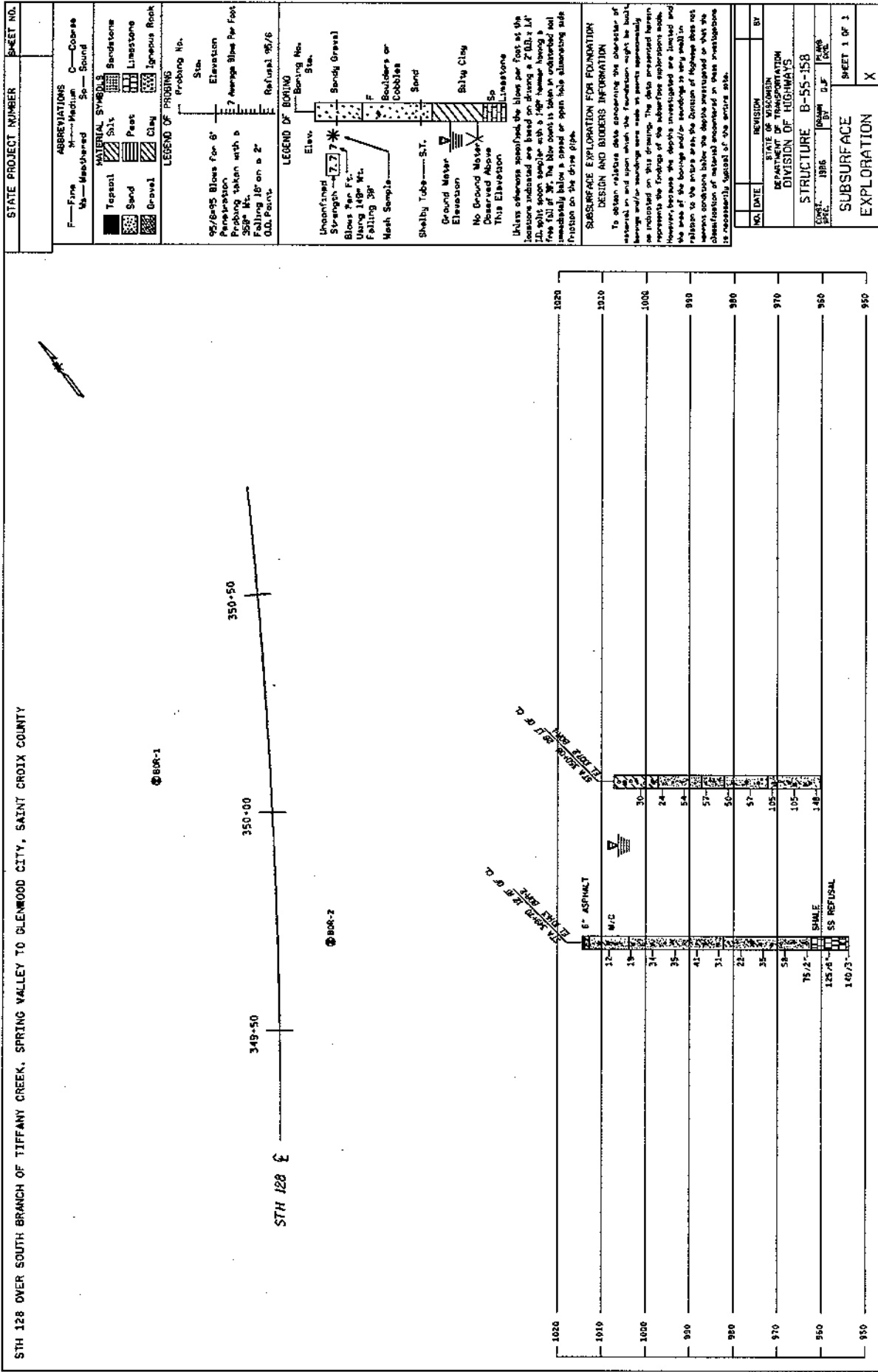
No unusual, unique or especially difficult construction problems are foreseen.

8. RECOMMENDATIONS

- A. Use H-piles or oil field pipe piles driven to rock/weathered rock at 9000 psi load in the steel section. The pile tip elevation should be $965 \pm$.
- B. Use a good sand material for fill and back fill.

m00174

STH 128 OVER SOUTH BRANCH OF TIFFANY CREEK, SPRING VALLEY TO GLENWOOD CITY, SAINT CROIX COUNTY



FIELD BORING LOG

EL3(S) 385

State of Wisconsin/Department of Transportation

Boring No. 1 Structure B-55-158 County St Croix Sheet 1 of 2
 Project 8651-00-00 Road STH 128 Spring Valley - Glenwood City
 Station 350+08 Offset 26' LT Existing Surface Elevation 1007

GROUND WATER OBSERVATIONS

While drilling 1.2' Time after drilling _____
 Before casing removal _____ Depth to water _____
 After Boring Completed _____ Depth to cave-in _____
 Cave In _____ Water Notes _____

MOISTURE
 D = Damp
 M = Moist
 W = Wet
 HS = Hollowstem
 WA = Washhead
 RB = Rockbit
 ST = Shelby tube
 SS = Split spoon
 DM = Drilling mud
 DRILLING METHOD
 A = Auger
 C = Coring
 W = Wash
 E = Easy
 M = Medium
 H = Hard
 Start 3-31-94 Unit 7
 Finish 4-5-94 Chief Horstman

Sample No.	Moisture	Blows on Sampler		Sample and Recovery	Visual Field Classification and Remarks	Unconfined Strength	Boulders	Blows on			Drilling Method
		0/6	6/12					Casing Size	Probe Size	Size	
					Water Elev. 1006.2						
					Loose grey sandy SILT tr gravel						A
					tr cobbles						
1	W	10	14								
		16	20		Drove rock						
2	W	7	10								
		14	12		Firm Br SAND little gravel						
3	W	12	26								
		28	30		V. Dense						
4	W	22	33								
		24	18		V. DENSE Br SAND SOME GRAVEL						
5	W	22	25								
		25	21		V. DENSE Br SAND TR GRAVEL						
6	W	23	26								
		31	30								
7	W	36	45								
		60	39		V. DENSE Br SAND SOME GRAVEL						
					TR SILT						
8	W	90	60								
		45	51								

Checked by _____ Final _____ Boring No. 1

FIELD BORING LOG

EL3(S) 385

State of Wisconsin/Department of Transportation

Boring No. 1 Structure B-55-00-00 158 County SE CROIX Sheet 2 of 2
 Project 8651-00-00 Road STH "128" SPRING VALLEY - GLENWOOD CITY
 Station 350 + 08 Offset 26' LT EXISTING & Surface Elevation 1007.2

GROUND WATER OBSERVATIONS

While drilling 1.2' Time after drilling _____
 Before casing removal _____ Depth to water _____
 After Boring Completed _____ Depth to cave-in _____
 Cave In _____ Water Notes _____

MOISTURE
 D = Damp
 M = Moist
 W = Wet

HS = Hollowstem
 WA = Washahead
 RB = Rockbit

ST = Shelby tube
 SS = Split spoon
 DM = Drilling mud

A = Auger
 C = Coring
 W = Wash

E = Easy
 M = Medium
 H = Hard

Start 3-3-94 Unit 7
 Finish 4-5-94 Chief Anderson

Sample No.	Moisture	Blows on Sampler		Sample and Recovery	VISUAL FIELD CLASSIFICATION AND REMARKS	Unconfined Strength	Boulders	Blows on		Drilling Method
		0/6	6/12					Casing Size	Probe Size	
8	W	90 48	60 51	40	Water Elev. 1006.2 VERY DENSE BR SAND some GRAVEL TR SILT.					WX RG revers
9	W	91 73	75 60	45						
				50						
				55						
				60						
				65						
				70						
				75						
				80						

Checked by _____ Final _____ Boring No. 1

FIELD BORING LOG

BL3(S) 385

State of Wisconsin/Department of Transportation

Boring No. 2 Structure B-55-158 County St Croix Sheet 1 of 2
Project 8651-00-00 Road STH 128 Spring Valley - Glenwood City
Station 349+70 Offset 12' Rt Existing E Surface Elevation 1014.3

GROUND WATER OBSERVATIONS

While drilling _____ Time after drilling _____
Before casing removal _____ Depth to water _____
After Boring Completed _____ Depth to cave-in _____
Cave In _____ Water Notes _____

MOISTURE D = Damp M = Moist W = Wet
HS = Hollowstem WA = Washahead RB = Rockbit
ST = Shelby tube SS = Split spoon DM = Drilling mud
A = Auger C = Coring W = Wash
E = Easy M = Medium H = Hard
Start 4-6-94 Unit 4
Finish _____ Chief Horstman

Sample No.	Moisture	Blows on Sampler		Sample and Recovery	VISUAL FIELD CLASSIFICATION AND REMARKS	Unconfined Strength	Boulders	Blows on		Drilling Method
		0/6	6/12					Casing 3"	Probe Size	
					<u>Asphalt</u>					
					<u>Br. SAND some gravel to silt</u>					<u>A</u>
					<u>Firm Br. med. coarse SAND</u>					
					<u>little gravel to silt</u>					
1	M	5	6					7		
		6	7					10		
								12		
								18		
								22		
								24		
2	W	8	11		<u>Firm grey SAND</u>			<u>14</u>		<u>WA</u>
		10			<u>some gravel to silt</u>			12		<u>RB</u>
								15		<u>revert</u>
								17		
								21		
3	W	19	15					<u>17</u>		
		12								
4	W	17	18							
		20								
5	W	19	22							
		23								
6	W	15	16							
		17			<u>Firm Br. SAND to gravel + silt</u>					
7	W	9	13							
		14								
8		13	22							

Checked by 24 Final _____ Boring No. 2

FIELD BORING LOG

EL3(S) 385

State of Wisconsin/Department of Transportation

Boring No. 2 Structure B-55-158 County St Croix Sheet 2 of 2

Project 8651-00-00 Road STH 128 Spring Valley - Glenwood City

Station	349+70	Offset	12' Rt Existing ϕ	Surface Elevation	1014.3
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GROUND WATER OBSERVATIONS

While drilling _____ Time after drilling _____

Before casing removal _____ Depth to water _____

After Boring Completed _____ Depth to cave-in _____

Cave In _____ Water Notes _____

MOISTURE	DRILLING METHOD	Start 4-6-94 Unit 4
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D = Damp
M = Moist

W = Wet

...the ...

RB = Rockbit

Blows on	pu	
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Blows on Sampler

DM = Drilling mud

Trial	Control	MCI	AD
1	95	85	75
2	95	85	75
3	95	80	70
4	95	78	68
5	95	75	65

1. *Chlorophyll a* (Chl *a*)

W = Wash

Condition	Control (%)	MCI (%)	AD (%)
A	~95	~85	~75
B	~90	~80	~70
C	~85	~75	~65
D	~95	~90	~85

..

H = Hard

100

Start: _____ End: _____

Finish Chief *Horizma*

Blows on

Sample No.	Moisture	Blows on Sampler		Sample and Recovery	VISUAL FIELD CLASSIFICATION AND REMARKS	Unconfined Strength	Boulders	Blows on			Drilling Method
		0/6	6/12					Casing Size	Probe Size		
8	w	13 24	13 22		V. Dense Br. SAND some gravel tr 5.18						WA RE rev
9	w	32 26	20 26								
		7 1/2"			S.S. refusal no recovery						
10	w	125			S.S. refusal						
11	w	140 31"			E.O.B.						

Checked by	Final	Boring No.
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Final

Boring No.